

FACULTY OF ENGINEERIGN AND SCIENCE

EXAM

Course code: ENE410

Course name: Fluid Dynamics and Hydro Power

Date: December 5, 2016

Duration: 4 hours

Number of pages incl. front page: 9

Resources allowed: All written and handwritten material, pocket calculator

Notes: 6 problems



EXAM ENE410 – Fluid Dynamics and Hydro Power

Problem 1

A recorded series of maximum values of instantaneous (peak) discharge is given in the table below. Knowing that <u>Gumbel's law</u> is the statistical law that best fits the data series, compute:

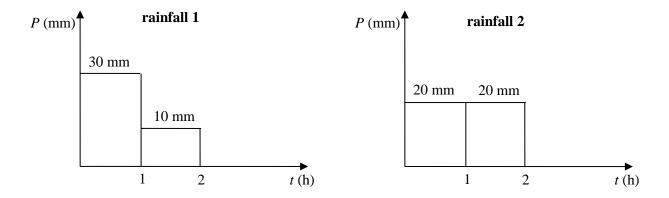
- a) the maximum peak discharge for the return period of 100 years;
- b) the return period corresponding to a peak discharge of 841.8 m³/s.

Year	Peak discharge (m³/s)
2002/03	1000
2002/06	950
2002/12	900
2002/05	800
2002/09	750
2002/08	700
2002/04	650
2002/07	600
2002/11	500
2002/10	300

A unit hydrograph corresponding to a unitary precipitation (1 mm) with duration equal to 1 hour is presented in the table.

UNIT HYDROGRAPH	
Time	Discharge
<i>t</i> (h)	$Q (m^3/s)$
0	0
1	10
2	20
3	15
4	10
5	5
6	0

Two rainfall events (rainfall 1 and 2) with same total duration (2 hours) and amount of precipitation (40 mm) have the following temporal distributions.



Determine which rainfall event originates the maximum peak discharge.



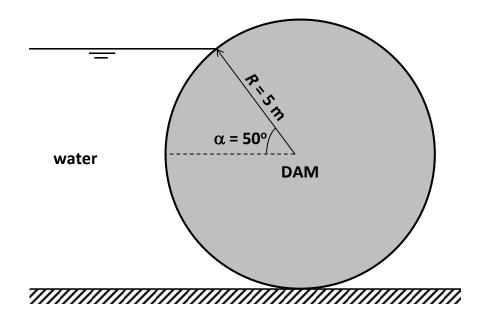
A hydropower plant uses, under a constant net head of 30.5 m, a discharge between 8.5 m 3 /s and 28.3 m 3 /s. In these conditions, determine:

- a) the most appropriate turbine and the minimum operating efficiency;
- b) the turbine main characteristics $(n_s, n, p \text{ and } P_{turbine})$.



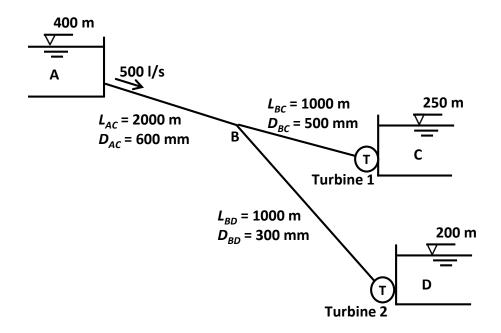
A cylindrical inflatable rubber dam with 5 m radius and 20 m wide is installed on a river, creating a water reservoir on its left side and having no water on the right side (see figure). In these conditions determine:

- a) the horizontal impulsion (hydrostatic force) on the dam;
- b) the vertical impulsion (hydrostatic force) on the dam.



Reservoir A can supply turbines 1 and 2 (both with <u>efficiency equal to 80%</u>). Turbine 1 discharges into reservoir C and turbine 2 discharges into reservoir D. All pipes are made of cast iron $(Q = 30.4D^{2.633}S_f^{0.51})$. All information concerning the reservoir levels and the length and diameter of the conduits are given in the figure. The <u>discharge from reservoir A is equal to 500 l/s</u>. Neglecting the minor head losses and assuming $\alpha = 1$, determine:

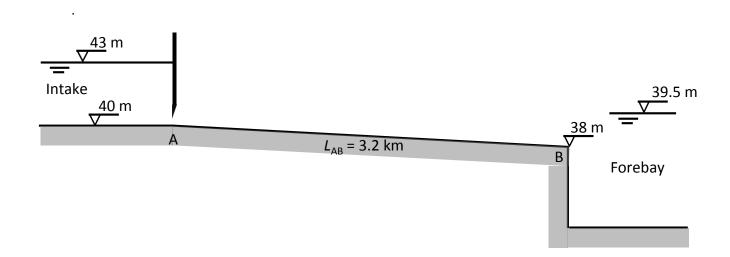
- a) the net head and power of turbine 1 when turbine 2 is not operating;
- b) the discharges used in turbine 1 and turbine 2 (i.e. Q_{BC} and Q_{BD}) for having a total power $P_{\text{turbine 1}} + P_{\text{turbine 2}} = 580 \text{ kW}$, maintaining the discharge from reservoir A equal to 500 l/s.





A hydropower system has a conveyance channel connecting the intake to a forebay, as shown in the figure. The discharge is controlled by a vertical sluice gate (A) in the intake. The gate has a contraction coefficient of 0.7. The levels in the intake and in the forebay are constant. The conveyance channel has a rectangular cross-section with 3 m width, is made of concrete ($n = 0.01 \text{ m}^{-1/3}\text{s}$) and has a length of 3.2 km, which is long enough to reach uniform flow conditions. In these conditions:

- a) determine the opening of the gate to have a discharge of 10 m³/s (assume that the flow depth downstream the gate is the one of the vena contracta, i.e. that if an hydraulic jump occurs downstream the gate it is free);
- b) verify if channel AB has mild or steep slope and if the assumption of a free hydraulic jump downstream the gate is correct;
- c) determine the flow depths upstream and downstream the hydraulic jump;
- d) <u>draw qualitatively the free-surface profile along the channel indicating the types</u> of gradually varying flow (GVF) curves.



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Formulae

$$\begin{split} \pi &= \gamma A h_0 & p_{alm}^{absolute} = 1.01325 \times 10^5 \text{ Pa} & \frac{p}{\gamma} + z = constant \\ X &= x_0 + \frac{I_{GG'}}{A x_0} & \pi_h = \gamma A_{proj} h_{0 proj} & \pi_v = \gamma \forall & \text{Re} = \frac{UD}{v} \\ \vec{M}_{in} - \vec{M}_{out} + \vec{I} + \vec{G} + \vec{\pi} = 0 & H = \frac{p}{\gamma} + z + \alpha \frac{U^2}{2g} & h_f = f \frac{L}{D} \frac{U^2}{2g} & h_f = S_f L \\ H_2 &= H_1 - h_f - h_m & \frac{1}{\sqrt{f}} = -2 \log_{10} \left(\frac{\varepsilon}{3.7D} + \frac{2.51}{\text{Re} \sqrt{f}} \right) & h_m = K \frac{U^2}{2g} \\ P_t &= \eta P_{flow} = \eta \gamma Q H_{net} & P_{pump} = \frac{P_{flow}}{\eta} = \frac{\gamma Q H_t}{\eta} & E = h + \alpha \frac{U^2}{2g} \\ h_c &= \sqrt[3]{\frac{Q^2}{gb^2}} & R_h = \frac{A}{P} & Q = \frac{1}{n} S R_h^{2/3} S_f^{1/2} & Q_0 = \sqrt{g} S(h_c) \sqrt{\frac{S(h_c)}{b(h_c)}} \\ Q &= C b \sqrt{2g} H^{3/2} & M = \alpha' \rho U^2 S = \alpha' \rho Q U = \alpha' \rho \frac{Q^2}{S} & h_{vc} = C h_{opening} & m = \frac{h_2'}{h_2} \\ T_X (\chi) &= \frac{1}{1 - F_X(\chi)} & \bar{x} = \sum_{i=1}^n x_i / n & \sigma_x = \sqrt{\sum_{i=1}^n (x_i - \bar{x})^2 / (n - 1)} & C_a = \frac{n \sum_{i=1}^n (x_i - \bar{x})^3}{(n - 1)(n - 2) \sigma_x^3} \\ P \cdot n = 60f & h_{s \max} = \frac{P_{atm}}{\gamma} - \frac{P_v}{\gamma} - \sigma H & \sigma = 0.625 \left(\frac{n_{s [m,kW]}}{380.78} \right)^2 & n_s = n \frac{P^{1/2}}{H^{5/4}} \end{split}$$

 $1 \, \text{ft} \approx 0.3048 \, \text{m}$

STATISTICAL LAWS		
NORMAL		
LOG-NORMAL (GALTON)	$x = e^{(\bar{y} + K_N \sigma_y)}$ $y = \ln(x)$ $y = \bar{y} + K_N \sigma_y$	
GUMBEL	$x = \bar{x} + K_G \sigma_x \qquad K_G = -\frac{\sqrt{6}}{\pi} \left(0.5772 + \ln \left[\ln \left(\frac{T}{T - 1} \right) \right] \right)$	
PEARSON III	$x = \overline{x} + K_{III}\sigma_{x}$ $K_{III} = K_{N} + \left(K_{N}^{2} - 1\right)\left(\frac{C_{a}}{6}\right) + \frac{1}{3}\left(K_{N}^{3} - 6K_{N}\right)\left(\frac{C_{a}}{6}\right)^{2} - \left(K_{N}^{2} - 1\right)\left(\frac{C_{a}}{6}\right)^{3} + K_{N}\left(\frac{C_{a}}{6}\right)^{4} + \frac{1}{3}\left(\frac{C_{a}}{6}\right)^{5}$	

